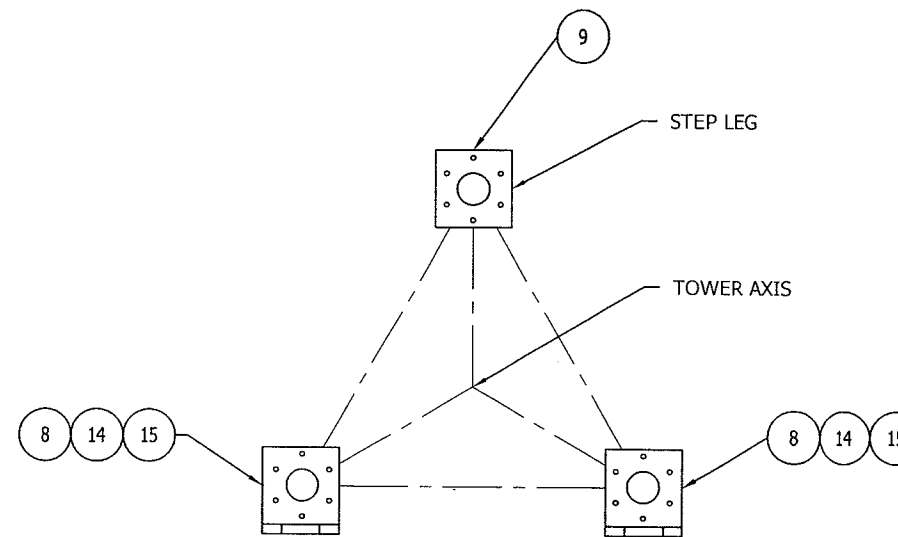
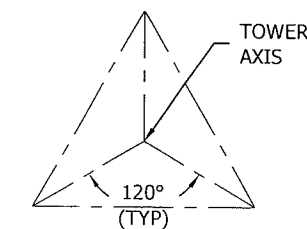


TOWER DESIGN LOADING		
DESIGN WIND LOAD PER ANSI/TIA/EIA-222-G: BASIC WIND SPEED (NO ICE) = 125 MPH BASIC WIND SPEED (ICE) = 60 MPH DESIGN ICE THICKNESS = 1.25 IN. STRUCTURE CLASS = II EXPOSURE CATEGORY = C TOPOGRAPHIC CATEGORY = 1 EARTHQUAKE SPECTRAL RESPONSE ACCELERATION, $S_s < 1.000$ THIS TOWER IS DESIGNED TO SUPPORT THE FOLLOWING LOADS:		
ELEVATION (FT)	ANTENNA TYPE	LINE SIZE (NOM)
TOP	VENTERA 10KW TURBINE ALLOWABLE THRUST = 1,000 LBS	(2) 1" CONDUIT

TOWER DESIGN IS BASED ON STATIC LOADING CONDITIONS ONLY. HARMONICS AND/OR DYNAMICS HAVE NOT BEEN CONSIDERED.



SECTION A-A



TOWER CONFIGURATION
N.T.S.

MAXIMUM FACTORED REACTIONS	
COMPRESSION	= 106.5 KIPS
TENSION	= 91.1 KIPS
TOTAL SHEAR	= 18.2 KIPS
O.T.M.	= 1128.7 FT-KIPS

ANCHOR MATERIAL				
ITEM	QTY	PART NO.	DESCRIPTION	DWG. NO.
1	1	18F1264H1	ANCHOR BOLT STR ASSY 18-1X48	18F1264H1
100093LAB - TOWER ACCESSORIES				
ITEM	QTY	PART NO.	DESCRIPTION	DWG. NO.
2	1	10N406	SECTION ASSY 10N 20' 3.5EH BASE	10N406
3	1	9N182	SECTION ASSY 9N 20' 3.5EH	A830385
4	1	8N111	SECTION ASSY 8N 20' 3EH	A800209
5	1	7N29	SECTION ASSY 7N 20' 2.5EH	A780168
6	1	6N69	SECTION ASSY 6N 20' 2.5STD	A790310
7	1	VG861A	SECTION ASSY VG1S 2.5STD S9	VG861A
8	2	VH069	HINGE BASE PLATE FOR 1" ANCHOR	N/A
9	1	VH070	HINGE BASE PLATE FOR 1" ANCHOR	N/A
10	3	BGK8GGX	KIT BASE GRD SSV 1"AB GALV.	B070997
11	1	ACWS	SIGN ANTI-CLIMB WARNING ASSY	N/A
12	1	B651264	STEPBOLT DETAIL	B651264
13	1	A790135	DRAWING BOLT ASSY	A790135
14	4	1CTLFG/2H	LOCK NUT 1 A563 DH HDG	N/A
15	2	290033	THREADED ROD 1(-8)X14.00 LG	N/A
16	1	KH8083	MOUNT TUBE 5.56ODX.258WX19.06'	N/A
17	1	ROHN-TAG	STANDARD ROHN TAG	N/A
18	1	710004	CARTON, NO. 80	N/A
19	1	DWG-0127	TOWER HINGE ASSEMBLY	DWG-0127

- ROHN PRODUCTS, LLC TOWER DESIGNS CONFORM TO ANSI/TIA/EIA-222-G UNLESS OTHERWISE SPECIFIED UNDER TOWER DESIGN LOADING.
- TURBINE AND LINES LISTED IN TOWER DESIGN LOADING TABLE ARE PROVIDED BY OTHERS UNLESS OTHERWISE SPECIFIED.
- THE DESIGN LOADING CRITERIA INDICATED HAS BEEN PROVIDED TO ROHN. THE DESIGN LOADING CRITERIA HAS BEEN ASSUMED TO BE BASED ON SITE-SPECIFIC DATA IN ACCORDANCE WITH ANSI/TIA/EIA-222-G AND MUST BE VERIFIED BY OTHERS PRIOR TO INSTALLATION.
- SEE INDIVIDUAL SECTION ASSEMBLY DRAWINGS FOR PART NUMBERS AND SECTION ASSEMBLY DETAILS.
- STEP BOLTS ARE PROVIDED ON ONE LEG ONLY FOR ALL TOWER SECTIONS.
- REFER TO THE LATEST REVISIONS OF THE DRAWINGS SHOWN IN THE BILL OF MATERIALS.
- PAL NUTS ARE PROVIDED FOR ALL TOWER AND ANCHOR BOLTS (SEE DWG. A790135).
- THE LEG PART NUMBER IS STAMPED AT THE BOTTOM OF EACH LEG OF EACH SECTION.
- DESIGN ASSUMES LEVEL GRADE AT TOWER SITE.
- WORK SHALL BE IN ACCORDANCE WITH ANSI/TIA/EIA-222-G, "STRUCTURAL STANDARDS FOR STEEL ANTENNA TOWERS AND ANTENNA SUPPORTING STRUCTURES".
- TOLERANCE ON TOWER STEEL HEIGHT IS EQUAL TO PLUS 1% OR MINUS 1/2%.
- PURCHASER SHALL VERIFY THE INSTALLATION IS IN CONFORMANCE WITH LOCAL, STATE, AND FEDERAL REQUIREMENTS FOR OBSTRUCTION MARKING AND LIGHTING.
- TOWER MEMBER DESIGN DOES NOT INCLUDE STRESSES DUE TO ERECTION SINCE ERECTION EQUIPMENT AND CONDITIONS ARE UNKNOWN. DESIGN ASSUMES COMPETENT AND QUALIFIED PERSONNEL WILL ERECT THE TOWER.
- DESIGN ASSUMES THAT, AS A MINIMUM, MAINTENANCE AND INSPECTION WILL BE PERFORMED OVER THE LIFE OF THE STRUCTURE IN ACCORDANCE WITH ANSI/TIA/EIA-222-G. THE INSPECTION INTERVAL FOR WIND TURBINE SUPPORT STRUCTURES SHALL NOT EXCEED 6 MONTHS.
- NUMBERS SHOWN IN BALLOONS DENOTE ITEM NUMBERS IN BILL OF MATERIAL.
- WIND TURBINE & ATTACHMENT HARDWARE TO BE SUPPLIED BY OTHERS.
- FOUNDATIONS SHALL BE DESIGNED TO SUPPORT THE CONDITIONS EXISTING AT THE SITE.
- TOWER ORIENTATION TO BE DETERMINED BY OTHERS.
- THE DESIGN OF THE REFERENCED STRUCTURE HAS BEEN BASED ON EQUIVALENT STATIC LOADING CONDITIONS PROVIDED BY THE TURBINE MANUFACTURER. THE TURBINE MANUFACTURER MUST APPROVE THE DESIGN FOR PROPER PERFORMANCE WITH THE INTENDED TURBINE CONSIDERING AS A MINIMUM, FATIGUE, HARMONICS AND DYNAMIC LOADING. ROHN DOES NOT ACCEPT RESPONSIBILITY AND PROVIDES NO WARRANTY FOR FATIGUE, HARMONICS, OR DYNAMIC LOADING RELATED ISSUES.

FILE NO. 100093			
REVISIONS			
REV.	DESCRIPTION	DWN	CHK APP
DWG REFERENCE			
PO BOX 5999 PEORIA, IL 61601-5999 TOLL FREE 800-727-ROHN			
THIS DRAWING IS THE PROPERTY OF ROHN. IT IS NOT TO BE REPRODUCED, COPIED OR TRACED IN WHOLE OR IN PART WITHOUT OUR WRITTEN CONSENT.			
VENTERA ENERGY CORP. RSSV-130G ERECTION PROFILE 120 FT. SSV OVERALL HEIGHT 130 FT. GENERIC			
DWN:	LGC	CHK'D:	KTL
ENG'R:	HA	DATE:	Jun/02/2010
DRAWING NO:			REV:
100093-01-A1			0

Jun/15/2010 10:28:21 AM

Erection

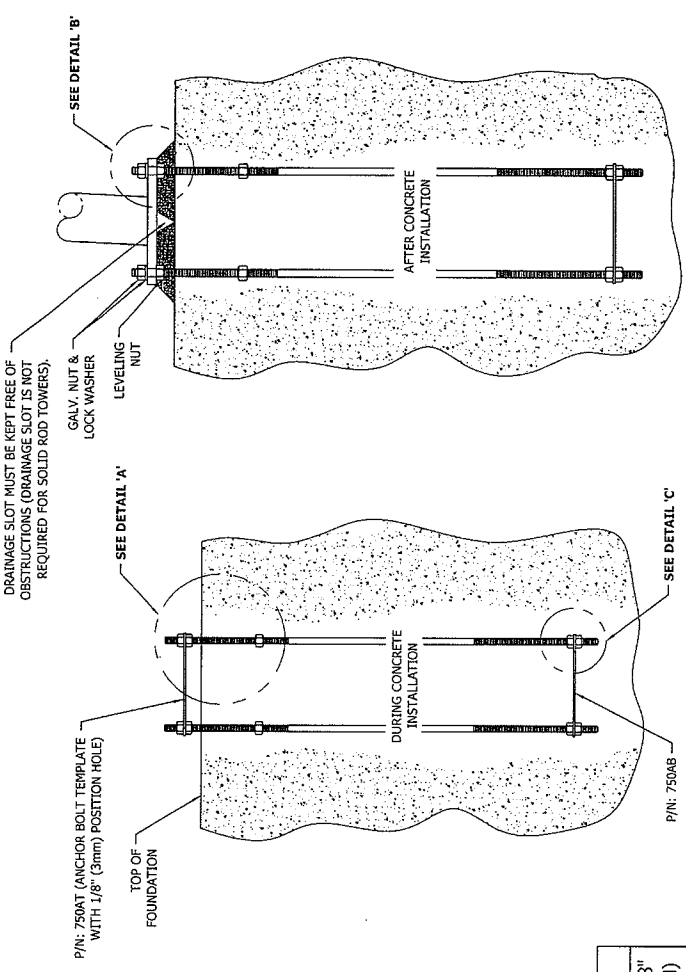
FILE NO.	100093
REVISIONS	
DESCRIPTION	DWN CHK APP

DWG REFERENCE	

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VENTERA ENERGY CORP. RSSV-130G
ANCHOR BOLT LAYOUT
1"Ø ANCHOR BOLTS (P/N: 18F1264H1)
GENERIC

DWN: LGC CHKD: KTL DATE: Jun/07/2010
ENGR: HA
DRAWING NO: 18F1264H1 REV: 0



DRAINAGE SLOT MUST BE KEPT FREE OF OBSTRUCTIONS (DRAINAGE SLOT IS NOT REQUIRED FOR SOLID ROD TOWERS).
GALV. NUT & LOCK WASHER
LEVELING NUT
SEE DETAIL 'B'
SEE DETAIL 'A'
TOP OF FOUNDATION
P/N: 750AT (ANCHOR BOLT TEMPLATE WITH 1/8" (3mm) POSITION HOLE)
DURING CONCRETE INSTALLATION
AFTER CONCRETE INSTALLATION
P/N: 750AB
SEE DETAIL 'C'

(6) 1" (25mm) DIA. X 48" (1219mm) L.G. ANCHOR BOLTS EQUALLY SPACED ON A 7'-1/2" (191mm) DIA. BOLT CIRCLE
TOWER AXIS
60°
30° TYP.
120° TYP.
E
D
A
C
B
E

PLAN VIEW N.T.S.				
A	B	C	D	E
12'-7 5/8" (3.851M)	10'-11 5/16" (3.335M)	7'-3 9/16" (2.224M)	12'-1 1/8" (3.686M)	12'-7 5/8" (3.851M)

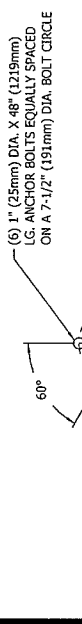
ANCHOR BOLT INSTALLATION TOLERANCES

- FACE SPREAD DIMENSION CENTER-TO-CENTER OF ANCHOR BOLT CIRCLES - PLUS OR MINUS 1/16" (2mm) OR 1/16" (2mm) PER 20 FT. (6m) OF FACE SPREAD
- MAXIMUM DIFFERENCE BETWEEN ANY TWO FOUNDATION ELEVATIONS - 1/2" (13mm)
- CONCRETE DIMENSIONS - PLUS OR MINUS 1" (25mm)
- DEPTH OF FOUNDATION - PLUS 3" (76mm) OR MINUS 0"
- DRILLED FOUNDATIONS OUT OF PLUMB - 1.0 DEGREE
- REINFORCING STEEL PLACEMENT - PER A.C.I. 301
- PREDICTION OF EMBEDMENTS - PLUS OR MINUS 1/8" (3mm)
- VERTICAL EMBEDMENTS OUT OF PLUMB - 1/2 DEGREE
- MAXIMUM DISTANCE FROM CENTERLINE OF ANCHOR BOLTS TO CENTERLINE OF FOUNDATION - 1/24 OF PIER DIAMETER UP TO A MAXIMUM OF 2" (50mm)
- ANCHOR BOLT SPACING - 1/16" (2mm)
- ANCHOR BOLT CIRCLE ORIENTATION - 1/4 DEGREE
- ANCHOR BOLT CIRCLE DIAMETER - PLUS OR MINUS 1/16" (2mm)

WARNING III
ENSURE DIMENSION 'D' & 'E' IS CORRECT ON ALL FACES PRIOR TO PLACING CONCRETE
AFTER ANCHOR BOLTS ARE INSTALLED AND CONCRETE HAS TAKEN ITS INITIAL SET, ANCHOR BOLTS MUST NOT BE MOVED, BENT OR REALIGNED IN ANY MANNER. A NUT LOCKING DEVICE MUST BE INSTALLED ON ALL ANCHOR BOLTS.

ELEVATION VIEWS
GROUT MAY BE ELIMINATED IF THE DISTANCE BETWEEN TOP OF CONCRETE AND BOTTOM OF LEVELING NUT IS LESS THAN OR EQUAL TO ANCHOR BOLT DIA.
1" MAX.
DETAIL 'B'
1/2" (13mm)
DETAIL 'C'

NOTES
1. ALL ANCHOR BOLTS MUST MEET OR EXCEED REQUIREMENTS OF A.S.T.M. F1554-S2, S5 GRADE 105.
2. GROUT TO BE 5000 PSI MIN. ULTIMATE STRENGTH/7 DAY NON-SHRINKING AND NON-METALLIC.
3. SPECIAL CARE MUST BE TAKEN WHEN LIFTING ANCHOR BOLT CLUSTER, IN ORDER TO PREVENT ANCHOR BOLT TEMPLATE DISTORTION.
4. ANCHOR BOLT ASSEMBLY MUST BE ADEQUATELY SUPPORTED AND RESTRAINED TO PREVENT MOVEMENT OF THE CLUSTER DURING CONCRETE INSTALLATION.
5. IT IS THE RESPONSIBILITY OF THE FOUNDATION CONTRACTOR TO VERIFY THAT THE CORRECT ANCHOR BOLT TEMPLATE AND FOUNDATION SHOWN ON RESPECTIVE SITE DRAWINGS ARE BEING USED.
6. IT IS THE RESPONSIBILITY OF THE FOUNDATION DESIGN ENGINEER TO INSURE THAT THE ANCHORAGES PROVIDED ARE COMPATIBLE WITH THE PROPOSED FOUNDATION DESIGN AND THAT THE CAPACITIES OF THE ANCHORAGES ARE NOT LIMITED BY THE STRENGTH OF THE FOUNDATIONS.

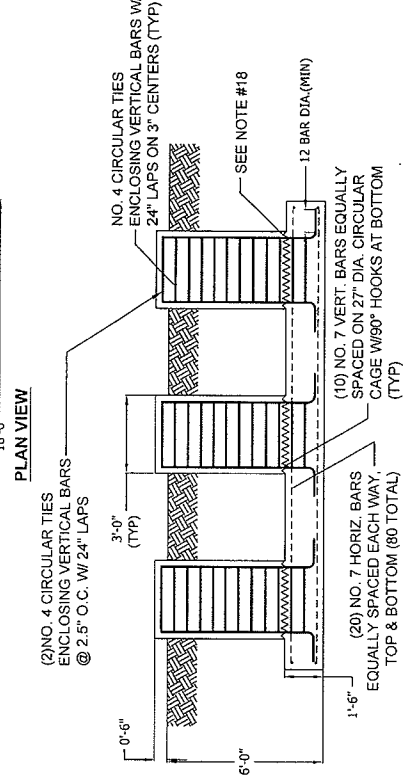
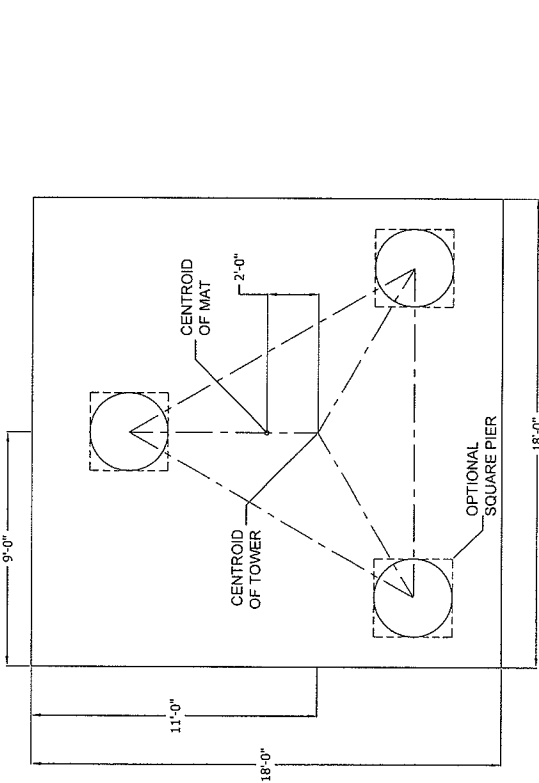


FILE NO.	100093
REVISIONS	
DESCRIPTION	
REV	DWN
CHK	APP

GENERAL NOTES

- A GEOTECHNICAL REPORT HAS NOT BEEN PROVIDED TO ROHN FOR FOUNDATION DESIGN. PURCHASER HAS REQUESTED FOUNDATION DESIGN TO BE BASED ON PRESUMPTIVE CLAY SOIL DESIGN PARAMETERS. IT IS THE RESPONSIBILITY OF THE PURCHASER TO VERIFY THAT PRESUMPTIVE SOIL DESIGN PARAMETERS ARE APPROPRIATE BASED UPON ACTUAL SOIL CONDITIONS. FOUNDATION DESIGN MODIFICATIONS MAY BE REQUIRED IN THE EVENT THE FOLLOWING DESIGN PARAMETERS ARE NOT APPLICABLE FOR THE SUBSURFACE CONDITIONS ENCOUNTERED.
 - ULTIMATE SOIL BEARING PRESSURE AT 6 FT DEPTH = 5,000 PSF.
 - GROUND WATER TABLE IS AT OR BELOW FOUNDATION DEPTH.
 - MAXIMUM FROST PENETRATION DEPTH LESS THAN FOUNDATION DEPTH.
- WORK SHALL BE IN ACCORDANCE WITH LOCAL CODES, SAFETY REGULATIONS AND UNLESS OTHERWISE NOTED, THE LATEST REVISION OF ACI 318, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE". PROCEDURES FOR THE PROTECTION OF EXCAVATIONS, EXISTING CONSTRUCTION AND UTILITIES SHALL BE ESTABLISHED PRIOR TO FOUNDATION INSTALLATION.
- CONCRETE MATERIALS SHALL CONFORM TO THE APPROPRIATE STATE REQUIREMENTS FOR EXPOSED STRUCTURAL CONCRETE.
- PROPORTIONS OF CONCRETE MATERIALS SHALL BE SUITABLE FOR THE INSTALLATION METHOD UTILIZED AND SHALL RESULT IN DURABLE CONCRETE FOR RESISTANCE TO LOCAL ANTICIPATED AGGRESSIVE ACTIONS. THE DURABILITY REQUIREMENTS OF ACI 318 CHAPTER 4 SHALL BE SATISFIED BASED ON THE CONDITIONS EXPECTED AT THE SITE. AS A MINIMUM, CONCRETE SHALL DEVELOP A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI IN 28 DAYS.
- MAXIMUM SIZE OF AGGREGATE SHALL NOT EXCEED SIZE SUITABLE FOR INSTALLATION METHOD UTILIZED OR 1/3 CLEAR DISTANCE BETWEEN REINFORCING. MAXIMUM SIZE MAY BE INCREASED TO 2/3 CLEAR DISTANCE PROVIDED WORKABILITY AND METHODS OF CONSOLIDATION SUCH AS VIBRATING WILL PREVENT HONEYCOMBS OR VOIDS.
- REINFORCEMENT SHALL BE DEFORMED AND CONFORM TO THE REQUIREMENTS OF ASTM A615 GRADE UNLESS OTHERWISE NOTED. SPLICES IN REINFORCEMENT SHALL NOT BE ALLOWED UNLESS OTHERWISE INDICATED.
- WELDING IS PROHIBITED ON REINFORCING STEEL AND EMBEDMENTS.
- MINIMUM CONCRETE COVER FOR REINFORCEMENT SHALL BE 3 INCHES (76 MM) UNLESS OTHERWISE NOTED. APPROVED SPACERS SHALL BE USED TO INSURE A 3 INCH (76 MM) MINIMUM COVER ON REINFORCEMENT.
- CONCRETE COVER FROM TOP OF FOUNDATION TO ENDS OF VERTICAL REINFORCEMENT SHALL NOT EXCEED 3 INCHES (76MM) NOR BE LESS THAN 2 INCHES (51MM).
- FOUNDATION DESIGN ASSUMES STRUCTURAL BACKFILL TO BE COMPACTED IN 8 INCH (200 MM) MAXIMUM LAYERS TO 98% OF MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT IN ACCORDANCE WITH ASTM D998. ADDITIONALLY, STRUCTURAL BACKFILL MUST HAVE A MINIMUM COMPACTED UNIT WEIGHT OF 100 POUNDS PER CUBIC FOOT (16 KN/M3).
- FOUNDATION INSTALLATION SHALL BE SUPERVISED BY PERSONNEL KNOWLEDGEABLE AND EXPERIENCED WITH THE PROPOSED FOUNDATION TYPE. CONSTRUCTION SHALL BE IN ACCORDANCE WITH GENERALLY ACCEPTED INSTALLATION PRACTICES.
- FOUNDATION DESIGN ASSUMES FIELD INSPECTIONS WILL BE PERFORMED TO VERIFY THAT CONSTRUCTION MATERIALS, INSTALLATION METHODS AND ASSUMED DESIGN PARAMETERS ARE ACCEPTABLE BASED ON CONDITIONS EXISTING AT THE SITE.
- FOR FOUNDATION AND ANCHOR TOLERANCES SEE DRAWING A810214.
- LOOSE MATERIAL SHALL BE REMOVED FROM BOTTOM OF EXCAVATION PRIOR TO CONCRETE PLACEMENT. SIDES OF EXCAVATION SHALL BE ROUGH AND FREE OF LOOSE CUTTINGS.
- CONCRETE SHALL BE PLACED IN A MANNER THAT WILL PREVENT SEGREGATION OF CONCRETE MATERIALS. INFILTRATION OF WATER OR SOIL AND OTHER OCCURRENCES WHICH MAY DECREASE THE STRENGTH OR DURABILITY OF THE FOUNDATION.
- CONCRETE PREFERABLY SHALL BE PLACED AGAINST UNDISTURBED SOIL. WHEN FORMS ARE NECESSARY, THEY SHALL BE REMOVED PRIOR TO PLACING STRUCTURAL BACKFILL.
- CONSTRUCTION JOINTS, IF REQUIRED AT THE BASE OF THE PIERS, MUST BE INTENTIONALLY ROUGHENED TO A FULL AMPLITUDE OF 1/4 INCH (6 MM). FOUNDATION DESIGN ASSUMES NO OTHER CONSTRUCTION JOINTS.
- TOP OF FOUNDATION OUTSIDE LIMITS OF ANCHOR BOLTS SHALL BE SLOPED TO DRAIN WITH A FLOATED FINISH. AREA INSIDE LIMITS OF ANCHOR BOLTS SHALL BE LEVEL WITH A SCRATCHED FINISH.
- EXPOSED EDGES OF CONCRETE SHALL BE CHAMFERED 3/4" X 3/4" (19MM X 19MM) MINIMUM.

NOTE: SEE STRUCTURE ASSEMBLY DRAWING FOR FOUNDATION LAYOUT AND ANCHORAGE EMBEDMENT DRAWING NUMBER.



CONCRETE VOLUME (cu.yds)	
PIER	3.9
PAD	5.0
TOTAL	18.0
TOTAL	21.9
TOTAL	23.0

REACTIONS	
Maximum O.T.M #	128.71 FT-K
Total Tower Wt =	8.33 KIPS
Total Shear =	18.17 KIPS
Max. Shear/Leg =	10.78 KIPS
Max. Tenz/Leg =	91.06 KIPS
Max. Comp./Leg =	106.48 KIPS

DWG REFERENCE

MODEL RSSV-130G



PO BOX 9999
PEORIA, IL 61601-9999
TOLL FREE 800-727-ROHN

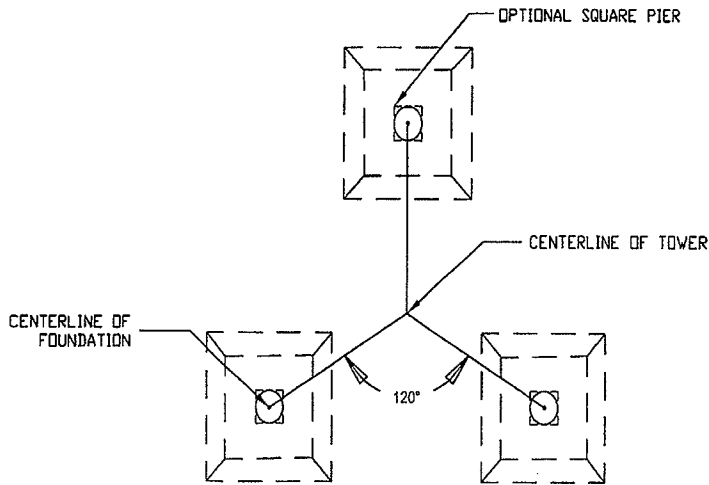
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VENTERA CORP. RSSV-130G
FOUNDATION
MAT WITH RAISED PIERS REV G CLAY
GENERIC

DWN: DWG
CHKD: HA
DATE: May/17/2010
ENGR:

DRAWING NO: 100093-01-F1
REV: 0

NOTE: SEE TOWER ASSEMBLY DRAWING FOR FOUNDATION LAYOUT AND ANCHORAGE EMBEDMENT DRAWING NUMBER.



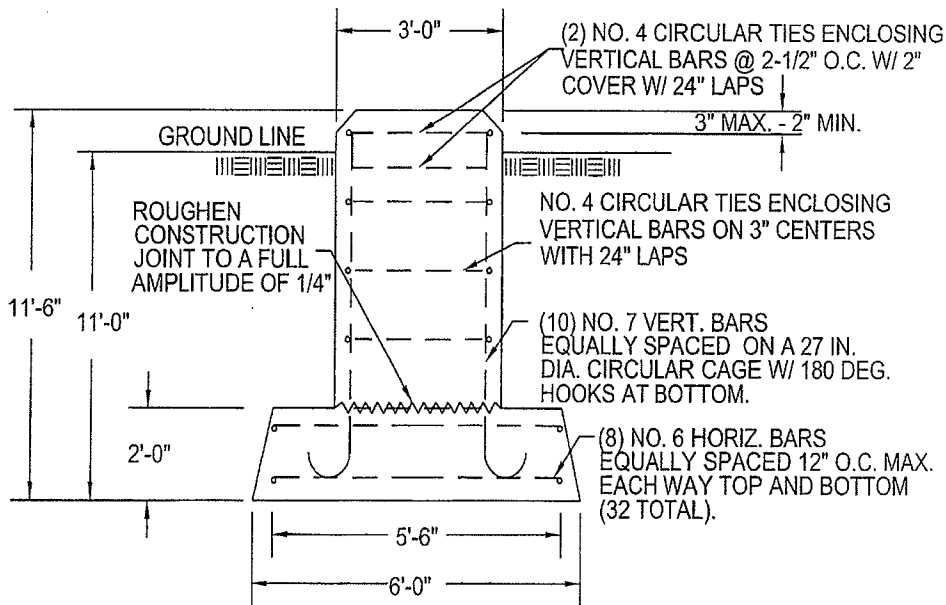
PLAN VIEW

CONCRETE VOLUME

(1) FOUNDATION 4.9 CU. YDS
 (3) FOUNDATIONS 14.7 CU. YDS

REACTIONS/LEG

DOWNLOAD = 106.5 KIPS
 UPLIFT = 91.1 KIPS
 SHEAR = 10.8 KIPS



ELEVATION VIEW

MODEL RSSV-130G

No.	Revision Description	Date	Rev By	Ckd By	Appd By
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Scale: NONE			ROHN PRODUCTS, LLC PEORIA, IL, USA PIER AND PAD FOUNDATION DETAILS PER ANSI/TIA-222-G PRESUMPTIVE SOIL - CLAY		
Drawn:	DWG	05/12/10			
Checked:	HA	5/13/10			
App. Eng.:	HA	5/13/10			
Parent File:		ENG. FILE: 100093	DWG. NO.:	100093-01-F2	REV.
			SHEET 1 OF 3		

Foundation General Notes

1. A geotechnical report has not been provided to ROHN for foundation design. Purchaser has requested foundation design to be based on presumptive soil design parameters. It is the responsibility of the purchaser to verify that presumptive soil design parameters are appropriate based upon actual soil conditions. Foundation design modifications may be required in the event the following design parameters are not applicable for the subsurface conditions encountered.
 - A. Uplift angle with vertical = 30.0 degrees.
 - B. Ultimate net bearing pressure at 11 foot depth = 5.0 ksf.
 - C. Ground water table at or below depth of foundation.
2. Work shall be in accordance with local codes, safety regulations and unless otherwise noted, the latest revision of ACI 318, "Building Code Requirements for Reinforced Concrete". Procedures for the protection of excavations, existing construction and utilities shall be established prior to foundation installation.
3. Concrete materials shall conform to the appropriate state requirements for exposed structural concrete.
4. Proportions of concrete materials shall be suitable for installation method utilized and shall result in durable concrete for resistance to local anticipated aggressive actions. The durability requirements of ACI 318 Chapter 4 shall be satisfied based on the conditions expected at the site. As a minimum, concrete shall develop a minimum compressive strength of 4000 psi (27.6 MPa) in 28 days.
5. Maximum size of aggregate shall not exceed size suitable for the installation method utilized or 1/3 clear distance behind or between reinforcing. Maximum size may be increased to 2/3 clear distance provided workability and methods of consolidation such as vibrating will prevent honeycombs or voids.
6. Reinforcement shall be deformed and conform to the requirements of ASTM A615 grade 60 unless otherwise noted. Splices in reinforcement shall not be allowed unless otherwise indicated.
7. Welding is prohibited on reinforcing steel and embeddings.
8. Minimum concrete cover for reinforcement shall be 3 inches (76 mm) unless otherwise noted. Approved spacers shall be used to insure a 3 inch (76 mm) minimum cover on reinforcement.
9. Concrete cover from top of foundation to ends of vertical reinforcement shall not exceed 3 inches (76mm) nor be less than 2 inches (51mm).
10. Foundation design assumes structural backfill to be compacted in 8 inch (200 mm) maximum layers to 95% of maximum dry density at optimum moisture content in accordance with ASTM D698. Additionally, structural backfill must have a minimum compacted unit weight of 100 lb./cu.ft. (15.7 kn/m3)..
11. Foundation installation shall be supervised by personnel knowledgeable and experienced with the proposed foundation type. Construction shall be in accordance with generally accepted installation practices.
12. Foundation design assumes field inspections will be performed to verify that construction materials, installation methods and assumed design parameters are acceptable based on

Foundation General Notes Continued

conditions existing at the site.

13. For foundation and anchor tolerances see structure assembly drawing.
14. Loose material shall be removed from bottom of excavation prior to concrete placement. Sides of excavation shall be rough and free of loose cuttings.
15. Concrete shall be placed in a manner that will prevent segregation of concrete materials, infiltration of water or soil and other occurrences which may decrease the strength or durability of the foundation.
16. Concrete preferably shall be placed against undisturbed soil. When forms are necessary, they shall be removed prior to placing structural backfill.
17. Construction joints, if required in piers, must be at least 12 inches (305 mm) below bottom of embedments and must be intentionally roughened to a full amplitude of 1/4 inch (6 mm). Foundation design assumes no other construction joints.
18. Top of foundation outside limits of anchor bolts shall be sloped to drain with a floated finish. Area inside limits of anchor bolts shall be level with a scratched finish.
19. Exposed edges of concrete shall be chamfered 3/4" x 3/4" (19mm x 19mm) minimum.

FILE NO. 100093

REVISIONS

DESCRIPTION

DWN CHK APP

REV

DESCRIPTION

DWN CHK APP

REV

DESCRIPTION

DWN CHK APP

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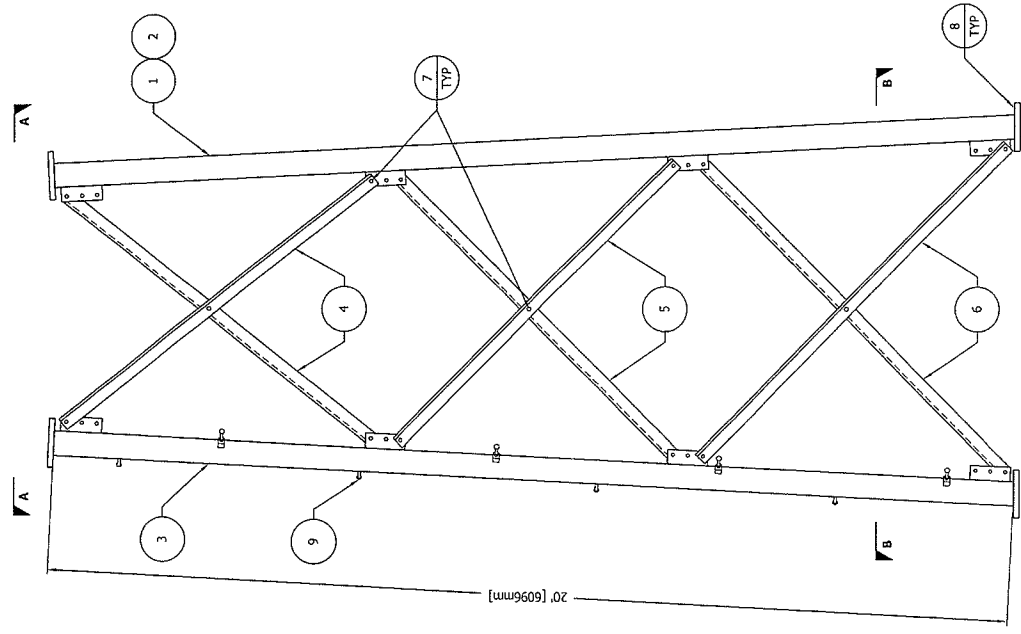
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REV

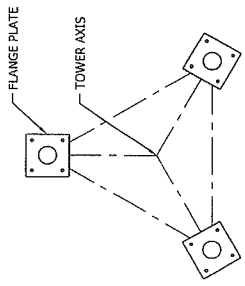
BILL OF MATERIALS			
ITEM	PART NO.	QTY	DESCRIPTION
1	V16142	1	LEG SSV 9-11IN 3.5EH 20' 7-10 [PIPE 89mm EH]
2	V16143	1	LEG SSV 9-11IN 3.5EH 20' 7-10 [PIPE 89mm EH]
3	V16144	1	LEG SSV 9-11IN 3.5EH 20' 7-10 [PIPE 89mm EH]
4	TN101	6	BRACE DS S510T L2.5X.19X12.29 [L 64X64X5]
5	TN102	6	BRACE DS S510T L2.5X.19X12.86 [L 64X64X5]
6	TN103	6	BRACE DS S510T L2.5X.19X13.35 [L 64X64X5]
7	210017GA	45	BOLT ASSY 1/2 X 1-3/4 HSB A325 [M13X32]
8	210053GA	18	BOLT ASSY 7/8 X 3-1/2 HSB A325 [M22X89]
9	5/8STEP	16	BOLT ASSY STEP 5/8X7 W/DBN [M16X178]

GENERAL NOTES:

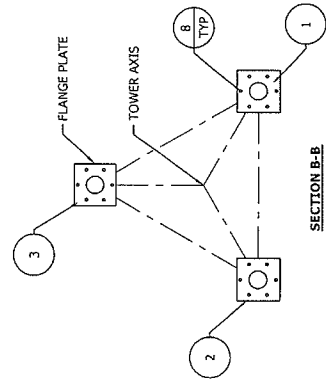
- LEG PART NUMBER IS STAMPED AT THE BOTTOM OF EACH LEG AND MUST BE LOCATED AT THE BOTTOM OF THE SECTION FOR PROPER ASSEMBLY.
- STEP BOLTS ARE PROVIDED ON ONE LEG ONLY.
- THIS SECTION IS A BASE SECTION. SEE THE TOWER ASSEMBLY FOR ANCHOR BOLTS. PAL NUTS ARE SUPPLIED FOR THE ANCHOR BOLTS.
- DRAWING IS IN U.S. AND IS FOR ASSEMBLY PURPOSES ONLY.
- NOMINAL METRIC EQUIVALENTS ARE GIVEN FOR REFERENCE ONLY AND SHALL NOT BE SUBSTITUTED FOR THE DESCRIBED SIZES UNLESS OTHERWISE APPROVED BY ROHN PRODUCTS.



ELEVATION VIEW



VIEW A-A



SECTION B-B

FLANGE	OFFSET	BEVEL	FLANGE PLATE (P/N)	SPREAD
TOP	N/A	N/A	7" X 7" X 1" (P/N: R-7F)	10'-7 5/8" [3242mm]
BOTTOM	N/A	3 1/3° STD	10" X 10" X 1" (SPEC)	12'-7 5/8" [3851mm]



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VENTERA ENERGY CORP. PSSV-130G
SECTION ASSEMBLY
DETAILS FOR SSV 10N406 (HINGED BASE)
GENERIC

DWN: LGC
CHKD: KTL
DATE: 10/17/2010

ENG: HA
DRAWING NO. 10N406

REV: 0

FILE NO. 100093

REVISIONS

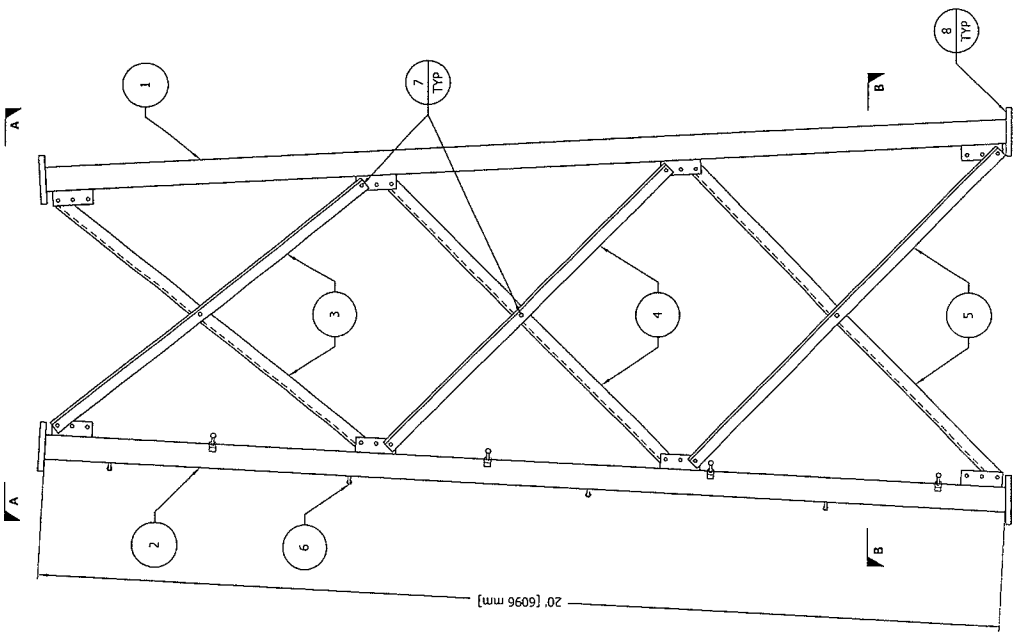
REV	DESCRIPTION	DWN	CHK	APP
1	REVISION BY AUTOCAD FORMAT	CHW	KTL	HA
1	DATE: 4/10/2010			

BILL OF MATERIAL

ITEM	PART NO.	QTY	DESCRIPTION
1	VL89	2	LEG SSV 9-1/4" X 3.5EH 20' 7 7/8" [PIPE 89mm EH]
2	VL89S	1	LEG SSV 9-1/4" X 3.5EH 20' 7 7/8" [PIPE 89mm EH]
3	VB379	6	BRACE DS S59T L2.5X.19X10.63" [L 64x64x5]
4	VB380	6	BRACE DS S59T L2.5X.19X11.17" [L 64x64x5]
5	VB381	6	BRACE DS S59T L2.5X.19X11.61" [L 64x64x5]
6	5/8STEP	16	BOLT ASSY STEP 5/8X7 W/DBN [M16x178]
7	210017GA	45	BOLT ASSY 1/2" X 1-1/4" HSB A325 [M13x32]
8	210063GA	12	BOLT ASSY 7/8" X 3-1/2" HSB A325 [M22x89]

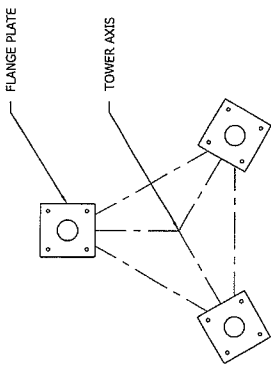
GENERAL NOTES:

- LEG PART NUMBER IS STAMPED AT THE BOTTOM OF EACH LEG AND MUST BE LOCATED AT THE BOTTOM OF THE SECTION FOR PROPER ASSEMBLY.
- STEP BOLTS ARE PROVIDED ON ONE LEG ONLY.
- FLANGE BOLTS ARE FOR FLANGE PLATES AT THE BOTTOM OF THE SECTION.
- DRAWING IS IN U.S. AND IS FOR ASSEMBLY PURPOSES ONLY.
- NOMINAL METRIC EQUIVALENTS ARE GIVEN FOR REFERENCE ONLY AND SHALL NOT BE SUBSTITUTED FOR THE DESCRIBED SIZES UNLESS OTHERWISE APPROVED BY ROHN PRODUCTS.

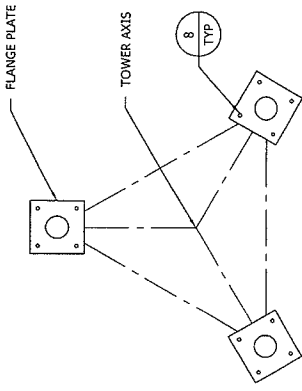


ELEVATION VIEW

FLANGE	OFFSET	BEVEL	FLANGE PLATE (P/N)	SPREAD
TOP	N/A	N/A	7" X 7" X 1" (P/N: R-7F)	8'-7 5/8" [2632mm]
BOTTOM	N/A	N/A	7" X 7" X 1" (P/N: R-7F)	10'-7 5/8" [3242mm]



VIEW A-A



SECTION B-B



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VENTERA ENERGY CORP. RSSV-130G
SECTION ASSEMBLY
DETAILS FOR SSV 9N182
GENERIC

DWN: B.F. CHKD: KTL DATE: NOV/27/1983

ENG: T.S. DRAWING NO: A830385

REV: 1

FILE NO. 100093

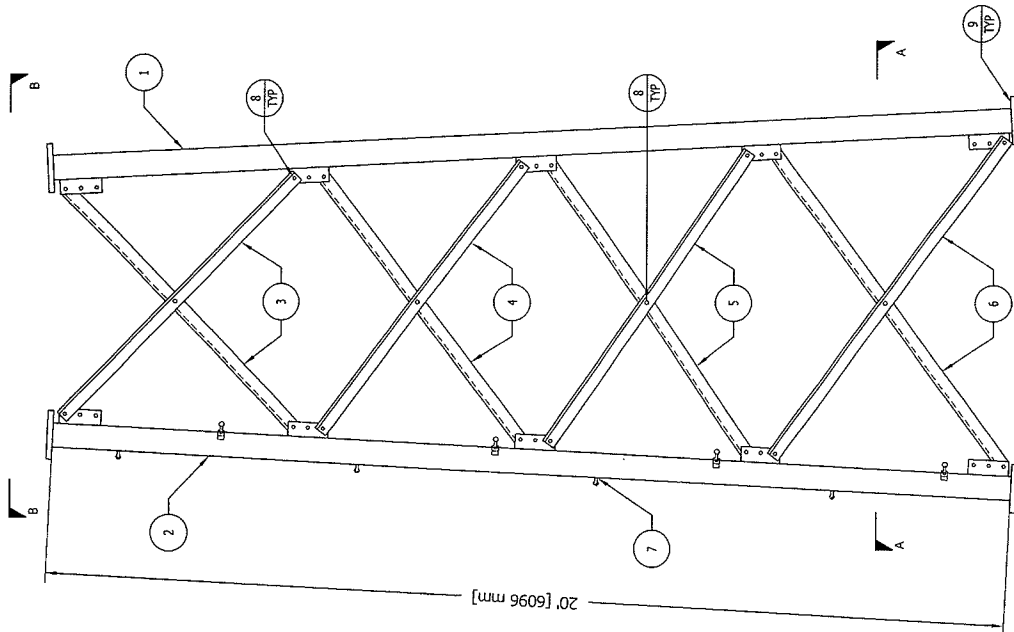
REVISIONS

REV	DESCRIPTION	DWN	CHK	APP
1	RE-DRAWN NEW AUTOCAD FORMAT.	ASD	RTL	HA
2	DATE: 04/29/2008 UPDATED TO NEW STANDARDS	CHW	RTL	HA
3	DATE: 04/22/2010			

ITEM	QTY	PART NO.	DESCRIPTION	DWG. NO.
1	2	VL651	LEG SSV 8N 3EH 206 7OFF S9	[PIPE 76mm EH]
2	1	VL651S	LEG SSV 8N 3EH 20 6 7OFF S9	[PIPE 76mm EH]
3	6	VB142	BRACE DS 5SBT L1.75X.13X7.95'	[L 44x44x3]
4	6	VB143	BRACE DS 5SBT L1.75X.13X8.36'	[L 44x44x3]
5	6	VB144	BRACE DS 5SBT L1.75X.13X8.79'	[L 44x44x3]
6	6	VB145	BRACE DS 5SBT L1.75X.13X9.10'	[L 44x44x3]
7	16	5/8STEP	BOLT ASSY STEP 5/8X7 W/DBN	[M16x178]
8	60	2100176A	BOLT ASSY 1/2 X 1-1/4 HSB A325	[M13x32]
9	12	210063GA	BOLT ASSY 7/8 X 3-1/2 HSB A325	[M22x89]

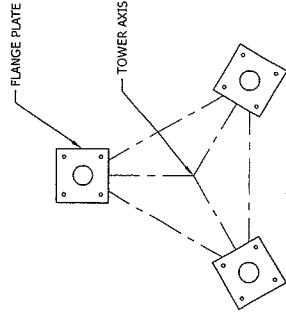
GENERAL NOTES:

1. LEG PART NUMBER IS STAMPED AT THE BOTTOM OF EACH LEG AND MUST BE LOCATED AT THE BOTTOM OF THE SECTION FOR PROPER ASSEMBLY.
2. STEP BOLTS ARE PROVIDED ON ONE LEG ONLY.
3. FLANGE BOLTS ARE FOR FLANGE PLATES AT THE BOTTOM OF THE SECTION.
4. DRAWING IS N.T.S. AND IS FOR ASSEMBLY PURPOSES ONLY.
5. NOMINAL METRIC EQUIVALENTS ARE GIVEN FOR REFERENCE ONLY AND SHALL NOT BE SUBSTITUTED FOR THE DESCRIBED SIZES UNLESS OTHERWISE APPROVED BY ROHN PRODUCTS.

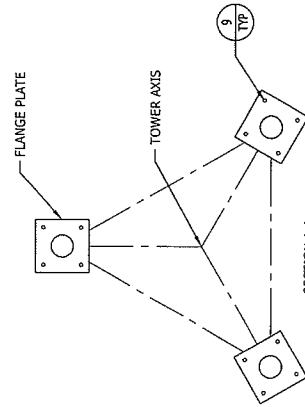


ELEVATION VIEW

FLANGE	OFFSET	BEVEL	FLANGE PLATE (P/N)	SPREAD
TOP	N/A	N/A	6" X 6" X 3/4" (P/N: R-6C)	6'-7 1/4" [2013mm]
BOTTOM	1/4" [6mm] STD	N/A	7" X 7" X 1" (P/N: R-7A)	8'-7 5/8" [2632mm]



VIEW B-B



SECTION A-A

DWG REFERENCE



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VENTERA ENERGY CORP. RSSV-130G
 SECTION ASSEMBLY
 DETAILS FOR SSV 8N111
 GENERIC

DWN: ASJ CHKD: JHD DATE: Jun/04/1980

ENGR: OH

DRAWING NO: A800209

REV: 2

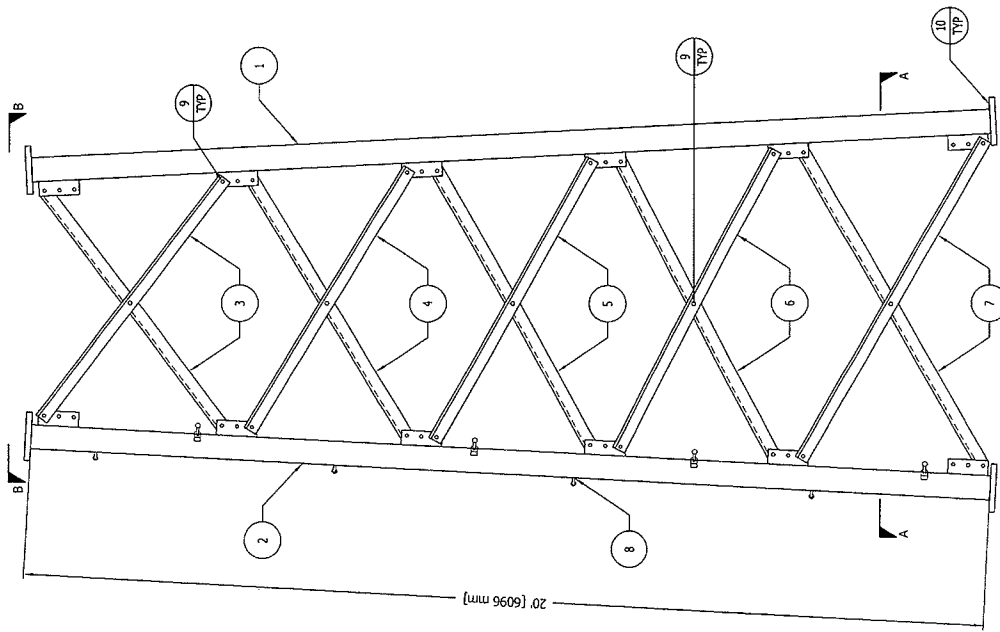
FILE NO. 100093

REVISIONS		DWN	CHK	APP
REV	DESCRIPTION	CHW	KTL	HA
2	UPDATED TO NEW STANDARDS			
	DATE: 04/13/2010			

ITEM	QTY	PART NO.	DESCRIPTION	DWG. NO.
1	2	VL133	LEG SSV 6-7N 2.5EH 20' 5 60F	[PIPE 76mm EH]
2	1	VL133S	LEG SSV 6-7N 2.5EH 20' 5 60F	[PIPE 76mm EH]
3	6	N71	BRACE DS S57T L1.5X13X6.71'	[L 38x38x3]
4	6	N72	BRACE DS S57T L1.5X13X6.03'	[L 38x38x3]
5	6	N73	BRACE DS S57T L1.5X13X6.35'	[L 38x38x3]
6	6	N74	BRACE DS S57T L1.5X13X6.69'	[L 38x38x3]
7	6	N75	BRACE DS S57T L1.5X13X6.92'	[L 38x38x3]
8	16	5/8STEP	BOLT ASSY STEP 5/8X7 W/DBN	[M16x178]
9	75	210017GA	BOLT ASSY 1/2 X 1-1/4 HSB A325	[M13x32]
10	12	210050GA	BOLT ASSY 3/4 X 2-3/4 HSB A325	[M19x70]

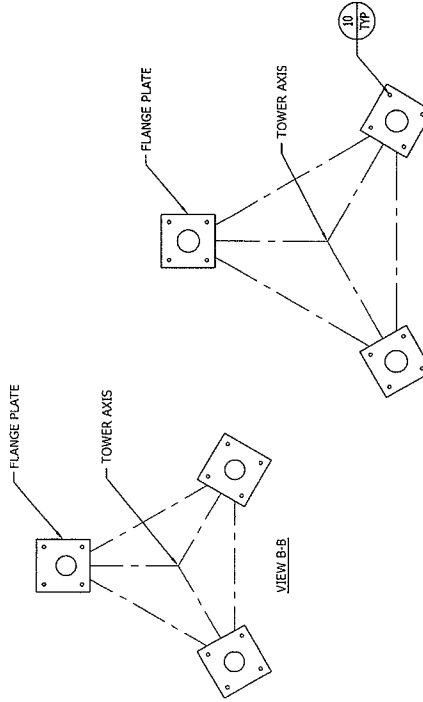
GENERAL NOTES:

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- STEP BOLTS ARE PROVIDED ON ONE LEG ONLY.
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ELEVATION VIEW

FLANGE	OFFSET	BEVEL	FLANGE PLATE (P/N)	SPREAD
TOP	N/A	N/A	5" X 5" X 3/4" (P/N: R-5C)	4'-6 3/4" [1391mm]
BOTTOM	1/4" [6mm] STD	N/A	6" X 6" X 3/4" (P/N: R-6A)	6'-7 1/4" [2013mm]



SECTION A-A

VIEW B-B



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VENTURA ENERGY CORP. RSSV-130G
 SECTION ASSEMBLY
 DETAILS FOR SSV 7N29
 GENERIC

DWN: AED CHKD: OH DATE: 5/21/11/1978

ENG'R: T.S

DRAWING NO: A780168
 REV: 2

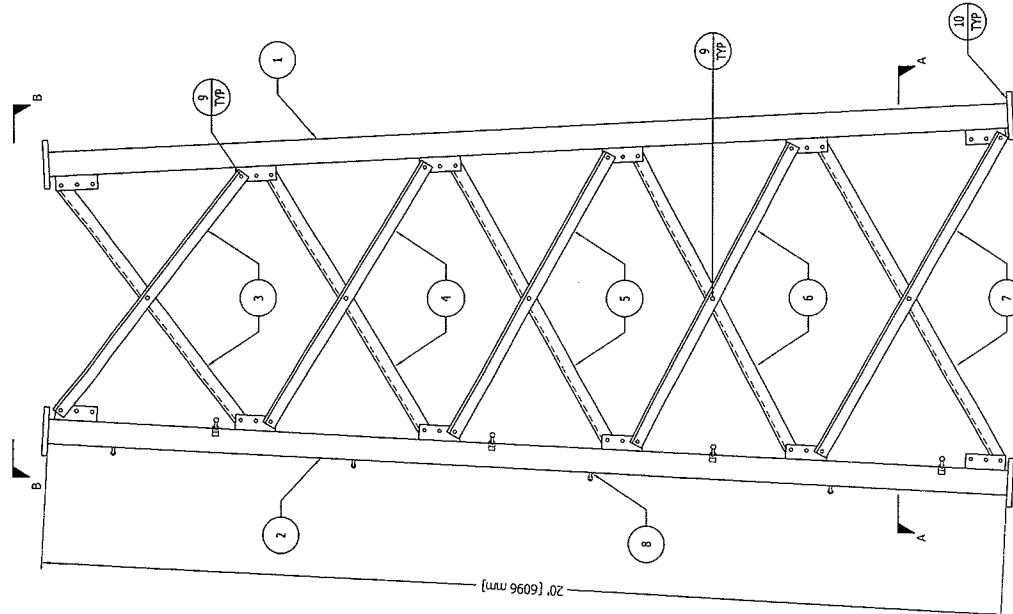
FILE NO. 100093

REV	DESCRIPTION	DWN	CHK	APP
1	REWORK TO NEW AUTOCAD FORMAT.	MB	JDH	H.A
2	DATE: 04/18/2005 REVISED VIEW #P	LCC	JDH	HA
3	DATE: 04/07/2008 UPDATED TO NEW STANDARDS	CHW	RTL	H.A
	DATE: 04/12/2010			

ITEM	QTY	PART NO.	DESCRIPTION	DWG. NO.
1	2	V649	LEG SSV 6-7N 2.5STD 20 5-5	[PIPE 64mmSTD]
2	1	V649S	LEG SSV 6-7N 2.5STD 20 5-5	[PIPE 64mmSTD]
3	6	N61	BRACE DS 5S6T L1.5X.13X4.39'	[L 38x38x3]
4	6	N62	BRACE DS 5S6T L1.5X.13X4.61'	[L 38x38x3]
5	6	N63	BRACE DS 5S6T L1.5X.13X4.86'	[L 38x38x3]
6	6	N64	BRACE DS 5S6T L1.5X.13X5.14'	[L 38x38x3]
7	6	N65	BRACE DS 5S6T L1.5X.13X5.28'	[L 38x38x3]
8	16	5/8STEP	BOLT ASSY STEP 5/8X7 W/DRN	[M16x178]
9	75	21007GA	BOLT ASSY 1/2 X 1-1/4 HSB A325	[M13x32]
10	12	210033GA	BOLT ASSY 5/8 X 2-1/2 HSB A325	[M16x64]

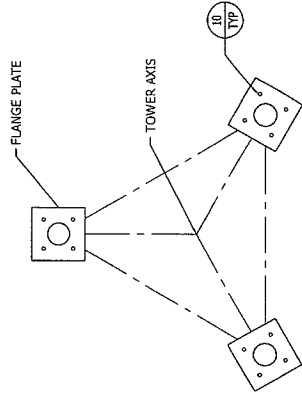
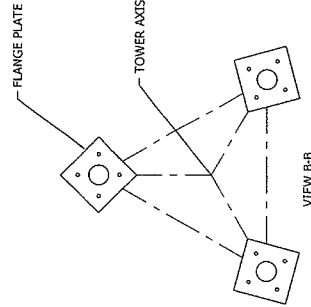
GENERAL NOTES:

- LEG PART NUMBER IS STAMPED AT THE BOTTOM OF EACH LEG AND MUST BE LOCATED AT THE BOTTOM OF THE SECTION FOR PROPER ASSEMBLY.
- STEP BOLTS ARE PROVIDED ON ONE LEG ONLY.
- FLANGE BOLTS ARE FOR FLANGE PLATES AT THE BOTTOM OF THE SECTION.
- DRAWING IS IN I.S. AND IS FOR ASSEMBLY PURPOSES ONLY.
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ELEVATION VIEW.

FLANGE	OFFSET	BEVEL	FLANGE PLATE (P/N)	SPREAD
TOP	N/A	3 1/2° REV	5" X 5" X 3/4" (P/N: R-5C)	2'-6 1/2" [775mm]
BOTTOM	N/A	N/A	5" X 5" X 3/4" (P/N: R-5C)	4'-6 3/4" [1391mm]



SECTION A-A



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VENTERA ENERGY CORP. RSSV-130G
SECTION ASSEMBLY
DETAILS FOR SSV 6N69
GENERIC

DWN:	AED	CHK'D:	RTL	DATE:	Dec/21/1979
ENGR:		TWS			
DRAWING NO.:	A790310				
REV:	3				

FILE NO. 100093

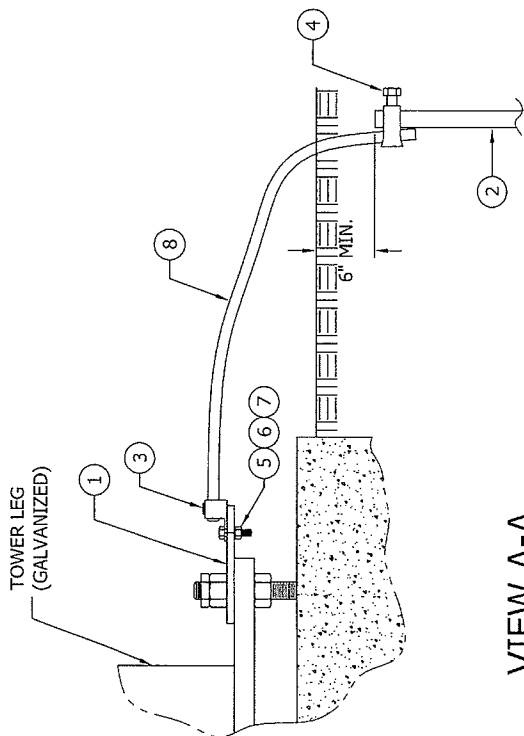
REV	DESCRIPTION	DWN	CHK	APP

ITEM	QTY	PART NO.	ITEM DESCRIPTION
1	1	SEE CHART	GROUND LUG
2	2	6260	ROD GROUND 5/8" X 10' COP CLAD
3	1	ADR25-21	LUG, GALVAN 6 - 250 ALCU
4	2	340016	CLAMP NO.8034 WB 3/4
5	1	220021	SCREW 1/4 X 1 HHMS SIL BRONZE
6	1	240006	NUT 1/4 HEX SILICONE BRONZE
7	2	250004	WASHER 1/4 FLAT SILICON
8	2	150400-30	7/16 6X25 EIP IWRC RRL GALV X 30'

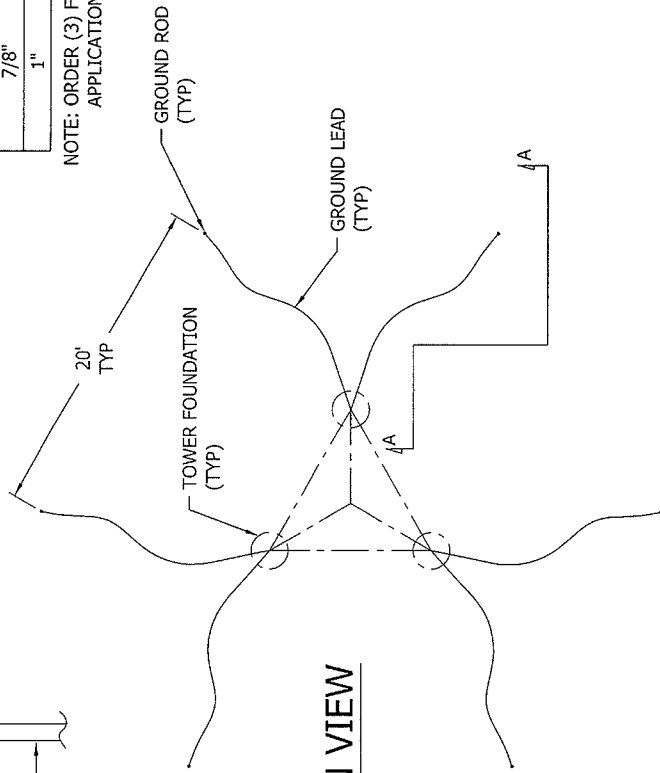
(B.O.M. SHOWN IS FOR ONE LEG ONLY)

ORDERING INFO	
ANCHOR ROD SIZE	ASSY. P/N
1/2"	R-BGK4GGX
5/8"	R-BGK5GGX
3/4"	R-BGK6GGX
7/8"	R-BGK7GGX
1"	R-BGK8GGX

NOTE: ORDER (3) FOR TYPICAL REV. G APPLICATIONS



VIEW A-A



PLAN VIEW

ANCHOR BOLT SIZE	GROUND PLATE
1/2"	GL4
5/8"	GL5
3/4"	GL6
7/8"	GL7
1"	GL8



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VENTERA ENERGY CORP. RSSV-130G
GROUNDING
TIA-222-G STANDARD - SS TWRS (GALV)
GENERIC

DWN: J.K	CHKD: JDM	DATE: 08/27/2007
ENGR:	DWG:	
DRAWING NO:	8070997	REV: 0

FILE NO.

100093

REVISIONS

REV.	DESCRIPTION	DWN	CHK	APP
1	DATE: Aug/01/2006			
2	DATE: Aug/01/2006			
3	DATE: Aug/01/2006			
4	DATE: Aug/01/2006			
5	DATE: Aug/01/2006			
6	UPDATED	JDM	HA	HA

DWG REFERENCE



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VENTERA ENERGY CORP. R5SV-130G

BOLT ASSEMBLY INSTALLATION
GENERIC

DWN: CH CHKD: DATE: Jul/05/1979

ENGR: TVS

DRAWING NO: A790135 REV: 6

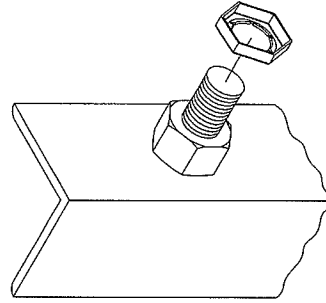
ASSEMBLY BOLT INSTALLATION

1. ALL STRUCTURE ASSEMBLY BOLTS ARE TO BE INSERTED OUT AND/OR UP, (EX. WITH NUTS AND PAL NUTS ON OUTSIDE OF TOWER FACE AND/OR ON TOP FLANGE PLATES) UNLESS PROHIBITED BY LACK OF CLEARANCE.
2. ALL ASSEMBLY AND ANCHOR BOLTS ARE TO BE TIGHTENED IN ACCORDANCE WITH ANSI/TIA/EIA-222-F 1996 SECTION 1.1.3.2 - (WHERE HIGH STRENGTH BOLTS ARE USED FOR BEARING-TYPE CONNECTIONS, AS A MINIMUM, THE BOLTS SHALL BE TIGHTENED TO A "SNUG TIGHT" CONDITION AS DEFINED IN THE NOVEMBER 13, 1985, AISC, "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS").
3. FLAT WASHERS ARE TO BE INSTALLED WITH BOLTS OVER SLOTTED HOLES.

CAUTION: DO NOT OVER TORQUE !! GALVANIZING ON BOLTS, NUTS AND STEEL PARTS MAY ACT AS A LUBRICANT, THUS OVER TIGHTENING MAY OCCUR AND MAY CAUSE BOLT TO CRACK AND SNAP OFF.

PAL NUT INSTALLATION

1. PAL NUTS ARE TO BE INSTALLED AFTER NUTS ARE TIGHT AND WITH EDGE LIP OUT (SEE PICTURE). PAL NUTS ARE NOT REQUIRED WHEN SELF-LOCKING NUTS OR LOCK WASHERS ARE PROVIDED.



FILE NO. 100093

REV	DESCRIPTION	DWN	CHK	APP

DWG REFERENCE



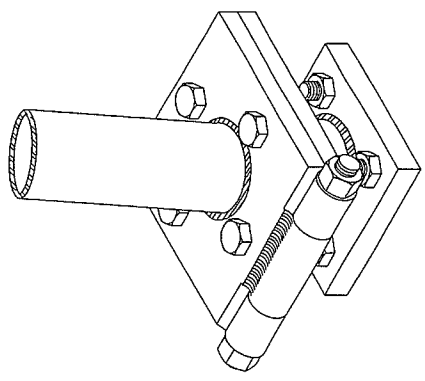
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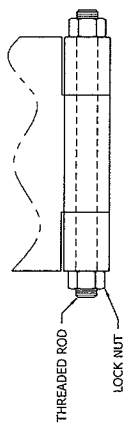
VENTERA ENERGY CORP. RSSV-130G
BASE PLATE
HINGE ASSEMBLY PROFILE
GENERIC

DWN:	M.F.	CHKD:	JDM	DATE:	July 11, 2010
ENGR:	HA				

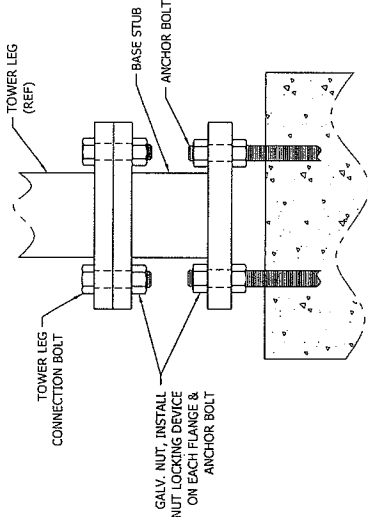
DRAWING NO. DWG-0127
REV: 0



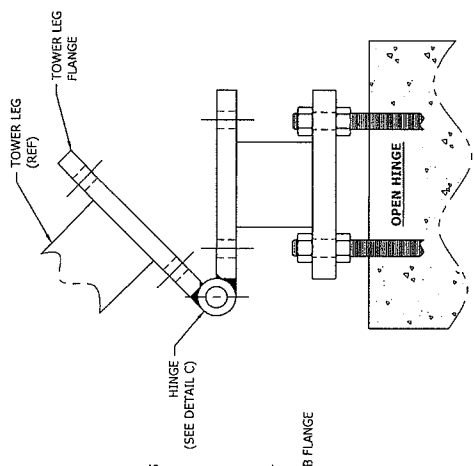
PICTORIAL VIEW



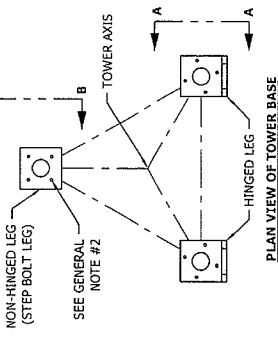
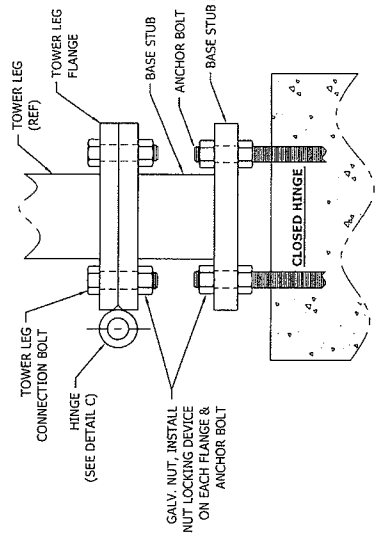
DETAIL C HINGE



VIEW B-B
NON-HINGED LEG



VIEW A-A
HINGED LEG



NOTE:
THE ANCHOR BOLT LAYOUT (NUMBER OF ANCHOR BOLTS, BOLT CIRCLE, ORIENTATION, ETC.) FOR A HINGED BASE KIT MAY BE DIFFERENT THAN THE ANCHOR BOLT LAYOUT FOR THE STRUCTURE WITHOUT A HINGED BASE. VERIFY ALL DIMENSIONS PRIOR TO CASTING ANCHOR BOLTS INTO A FOUNDATION.

GENERAL NOTES

- HINGED BASE KITS ARE AVAILABLE AS AN OPTION FOR ROHN MODEL SSV LATTICED SELF-SUPPORTING TOWERS. THEY ARE INTENDED FOR USE BY EXPERIENCED CONTRACTORS ONLY.
- DRAWING IS N.T.S. AND IS FOR GENERAL INFORMATION PURPOSES ONLY. NUMBER OF FLANGE AND ANCHOR BOLTS VARY BASED ON TOWER TYPE. REFER TO ANCHOR BOLT LAYOUT PROVIDED FOR A SPECIFIC TOWER FOR PROPER ANCHOR BOLT INFORMATION.
- THE MEANS AND METHODS OF RAISING AND LOWERING A TOWER MUST BE ESTABLISHED IN A CONTRACTOR'S RIGGING PLAN. ALL FORCES SHALL BE CONSIDERED FOR RIGGING, INCLUDING BUT NOT LIMITED TO THE ANTICIPATED WIND LOADING, THE WEIGHT OF THE TURBINE AND THE WEIGHT OF THE STRUCTURE AND ALL APPURTENANCES. RIGGING ATTACHMENTS SHALL NOT RESULT IN PERMANENT DEFORMATION OF ANY COMPONENT.
- ALL BASE FLANGE BOLTS MUST BE INSTALLED WHENEVER THE TOWER IS IN A SELF-SUPPORTED CONDITION. HINGE PINS MUST BE PROPERLY INSTALLED AND THE TOWER PROPERLY SUPPORTED IN ACCORDANCE WITH THE RIGGING PLAN PRIOR TO REMOVING ANY FLANGE BOLTS IN PREPARATION OF LOWERING THE TOWER.
- FLANGE BOLTS ARE A325 GALVANIZED BOLTS WHICH SHOULD NOT BE REUSED AFTER THEY HAVE BEEN TIGHTENED. ALL FLANGE BOLTS MUST BE REPLACED WITH A325 BOLTS OF THE SAME SIZE AND LENGTH AND INSTALLED PRIOR TO REMOVING THE TEMPORARY SUPPORT OF THE STRUCTURE. NUT LOCKING DEVICES MUST BE INSTALLED ON ALL FLANGE AND ANCHOR BOLTS.

WARNING: RAISING AND LOWERING A TOWER CAN BE A DANGEROUS OPERATION IF NOT PERFORMED IN ACCORDANCE WITH A RIGGING PLAN PREPARED BY AN EXPERIENCED CONTRACTOR. TOWERS MUST BE RAISED OR LOWERED BY EXPERIENCED CONTRACTORS ONLY.